

SPURLOCK STATION LANDFILL PHASE 3-B CELL CONSTRUCTION

CCR RULE POST-CONSTRUCTION DESIGN CERTIFICATION



EAST KENTUCKY POWER COOPERATIVE

COAL COMBUSTION RESIDUAL RULE COMPLIANCE

REV. 0 (10/23/2018)

Kenvirons, Inc.

CERTIFICATION

EAST KENTUCKY POWER COOPERATIVE SPURLOCK STATION LANDFILL – PHASE 3-B CELL CONSTRUCTION CCR RULE - POST-CONSTRUCTION DESIGN CERTIFICATION

CERTIFICATION

I hereby certify, as a Professional Engineer in the Commonwealth of Kentucky, that the composite liner and leachate collection and removal system has been constructed in accordance with the requirements of 40 CFR 257.70. The information in this document was assembled under my direct supervisory control. This report is not intended or represented to be suitable for reuse by East Kentucky Power Cooperative or others without specific verification or adaptation by the Engineer.

S. Tim Oakes, P.E. [21,483] - Kenvirons, Inc.

<u>Date:</u> 10/23/18

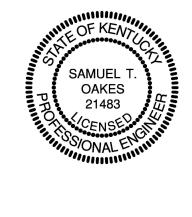


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1.0 INTRODUCTION

On April 17, 2015, the Environmental Protection Agency (EPA) issued the final version of the federal Coal Combustion Residual Rule (CCR Rule) to regulate the disposal of coal combustion residual (CCR) materials generated at coal-fired units. The rule will be administered as part of the Resource Conservation and Recovery Act [RCRA, 42 United States Code (U.S.C.) §6901 et seq.], under Subtitle D.

East Kentucky Power Cooperative (EKPC) is subject to the CCR Rule and as such will demonstrate compliance with 40 Code of Federal Regulations (CFR) §257.70(f). This document serves as EKPC's post-construction verification that the Phase 3-B lateral expansion was constructed in accordance with the project's design plans and specifications (composite liner system and leachate collection system) at Spurlock Station Landfill. The Phase 3-B lateral expansion was designed by AECOM and the construction quality assurance (CQA) for cell construction was certified by Kenvirons, Inc. Record drawings for the composite liner system and leachate collection system can be found in Appendix 1.

CONSTRUCTION CRITERIA								
Unit: Phase 3-B Cell Construction								
DESCRIPTION	CCR RULE COMPLIANCE							
DESCRIPTION	YES	NO	REPORT REFERENCE					
Composite Liner System ¹	\boxtimes		See Section 2.0					
Leachate Collection & Removal System			See Section 3.0					

TABLE 1-1 POST-CONSTRUCTION CERTIFICATION SUMMARY

¹ Certification of the composite liner system is based on the requirements of 40 CFR §257.70(f).

2.0 COMPOSITE LINER SYSTEM

The constructed composite liner system consists of two components: an upper component consisting of a 60-mil HDPE geomembrane and a lower component consisting of at least a two-foot compacted soil layer with a hydraulic conductivity of no more than 1×10^{-7} centimeters per second (cm/sec) over subgrade construction.

2.1 <u>Subgrade</u>

Rock/soil structural fill was taken from excavation activities within the cell and the borrow area directly to the northeast of the cell. Since the materials contained a large percentage of rock/gravel particles, no laboratory testing was performed. Structural fill

was used primarily for filling the lower elevations of the natural valley to achieve subgrade elevations while excavation was constructed primarily on the valley slopes. Structural fill was placed in 12-inch maximum lifts. All materials were bladed into place with a dozer and compacted with a sheeps foot compactor. Once proper grades were achieved, the cell subgrade was proof rolled utilizing a loaded Volvo A40 articulated dump truck. The proof roll exercise consisted of running the loaded dump truck across the subgrade surface. The ground surface was observed for indication of pumping or rutting. If signs of pumping or rutting were exhibited, the failed areas were reworked and proof roll tested until a passing test was observed. Once complete, the area tested was approved by the certifying Engineer for placement of the two-foot compacted soil layer.

An underdrain system consisting of pipe and gravel was installed beneath final subgrade elevations to intercept discontinuous seeps from underneath of Phase 3-B's composite liner system. The underdrain discharges intercepted flow to the sediment pond located southeast of Phase 3-B waste cell.

2.2 <u>Two-Foot Compacted Soil Layer</u>

The source of the compacted soil material was the soil borrow area west of the construction entrance off South Ripley Road and excavation within the cell. Processed material with particle sizes of two inches or less was used in the compacted soil layer. Samples of the compacted soil layer material were obtained and testing was performed to assure the material can achieve a hydraulic conductivity of no more than 1×10^{-7} cm/sec and to determine construction testing parameters. A standard Proctor density and optimum moisture content laboratory testing was performed to determine the construction testing parameters to achieve minimum field compaction of 92% of standard Proctor density and a target moisture content of -4% to +2% of optimum.

The compacted soil layer was placed on top of completed subgrade and is a minimum 24-inches thick throughout the cell. The soil liner material was spread into 6-inch compacted lifts using GPS-guided dozers to achieve proper grading. The lifts were compacted using a vibratory sheeps foot compactor. After compacting, the surface was rolled with a smooth drum vibratory roller. Moisture/density tests were taken on each lift using a nuclear density gauge at a frequency of no less than nine (9) tests per acre per lift.

If tested areas did not meet the minimum project requirements, that area was reworked and retested as necessary until retest results indicated compliance with project requirements. All lifts were scarified by tracking with dozers and water was added, if needed, prior to the next lift being placed. When the final lift was to grade, a smoothdrum, vibratory roller was used to prepare the compacted soil layer for geosynthetic installation. Prior to installation of geosynthetics, the compacted soil layer was inspected and approved by the certifying Engineer.

2.3 <u>60-mil HDPE Geomembrane</u>

Geomembrane deployment was monitored by Kenvirons, Inc. to insure that no damage was done to either the material or the soil liner and to ensure construction of the liner system was performed in accordance to the design and specifications for the project. The geomembrane material was deployed such that the panels and seams were approximately perpendicular to the contours of the slope. The panel alignment was adjusted by the Installer to provide orientation perpendicular to the contours and proper shingled overlap. In all cases, the FML panels were seamed on the day they were placed using a double hot-wedge, fusion welder. Each seam was observed by Kenvirons CQA monitor, with seam defects such as burn-throughs being marked for repair. All patches were heat tacked in place, ground for cleaning and to promote sufficient adhesion and then extrusion welded. Prior to seaming of the FML and again after approximately 4-hours of run time, trial welds were created per welding machine per welder operator each day. The trial seams were tested for peel and shear strength. For 60-mil Textured HDPE, the minimum peel strength for a fusion weld is 91 pounds per inch and 78 pounds per inch for extrusion welds. The minimum sheer strength for both extrusion and fusion welds is 120 pounds per inch. No panels were welded without the welder passing trial seam testing.

Non-destructive testing was performed on all fusion and extrusion welded panel seams and repairs. Air Pressure testing was conducted on fusion welded seams and vacuum box testing was performed on all extrusion welded seams including panel seaming and repair patches. Destructive samples were taken at selected locations for both fusion and extrusion welded seams. These samples were divided for testing on-site by the installation crew and the remaining sample sent out for independent laboratory testing.

3.0 LEACHATE COLLECTION & REMOVAL SYSTEM

The leachate collection system consists of a geocomposite drainage layer material, collection pipes, No. 57 peagravel drainage media and CoalTex geotextile.

3.1 <u>Geocomposite Drainage Layer</u>

The geocomposite was placed to provide sufficient overlap (approximately six inches) to tie the geonet and geotextile components together for each panel. The geonet component was joined via the installation of plastic pull ties placed a maximum of five feet apart on the longitudinal seams and a maximum of one foot apart on the cross seams. The top geotextile component was then fusion welded or heat bonded together. Additional geotextile material was heat bonded to the geocomposite to seal up all exposed geonet.

3.2 Leachate Collection Pipes

Leachate collection pipes consist of perforated 8-inch and 4-inch diameter HDPE, DR-17 pipes surrounded with No. 57 sized, low calcium carbonate content, washed rock and enclosed by a 14-oz/sy CCR compatible geotextile (CoalTex). The 4 inch pipes placed on the benches remain perforated and tie into the 8-inch leachate collection piping near the tie-in with previously constructed Area C Phase 1 and 2. The 8-inch perforated piping located along Phase 1 and 2's tie-in connects to existing Area C, Phase 3-A's leachate collection system. Cleanout risers for the collection pipes extend up the side slope of the cell to the north, south and east. Leachate gravity flows into the ditch east of the cell and finally into Pond 1.

3.3 <u>Geotextile</u>

The 14 oz/yd² geotextile (CoalTex) encasing the leachate collection piping and gravel was deployed with enough overlap (at least 6-inches) to connect the textile panels together by heat bonding or with zip ties. Geotextile patches were heat bonded where necessary to repair any cuts or tears in the geotextile.

4.0 **REPORT LIMITATIONS**

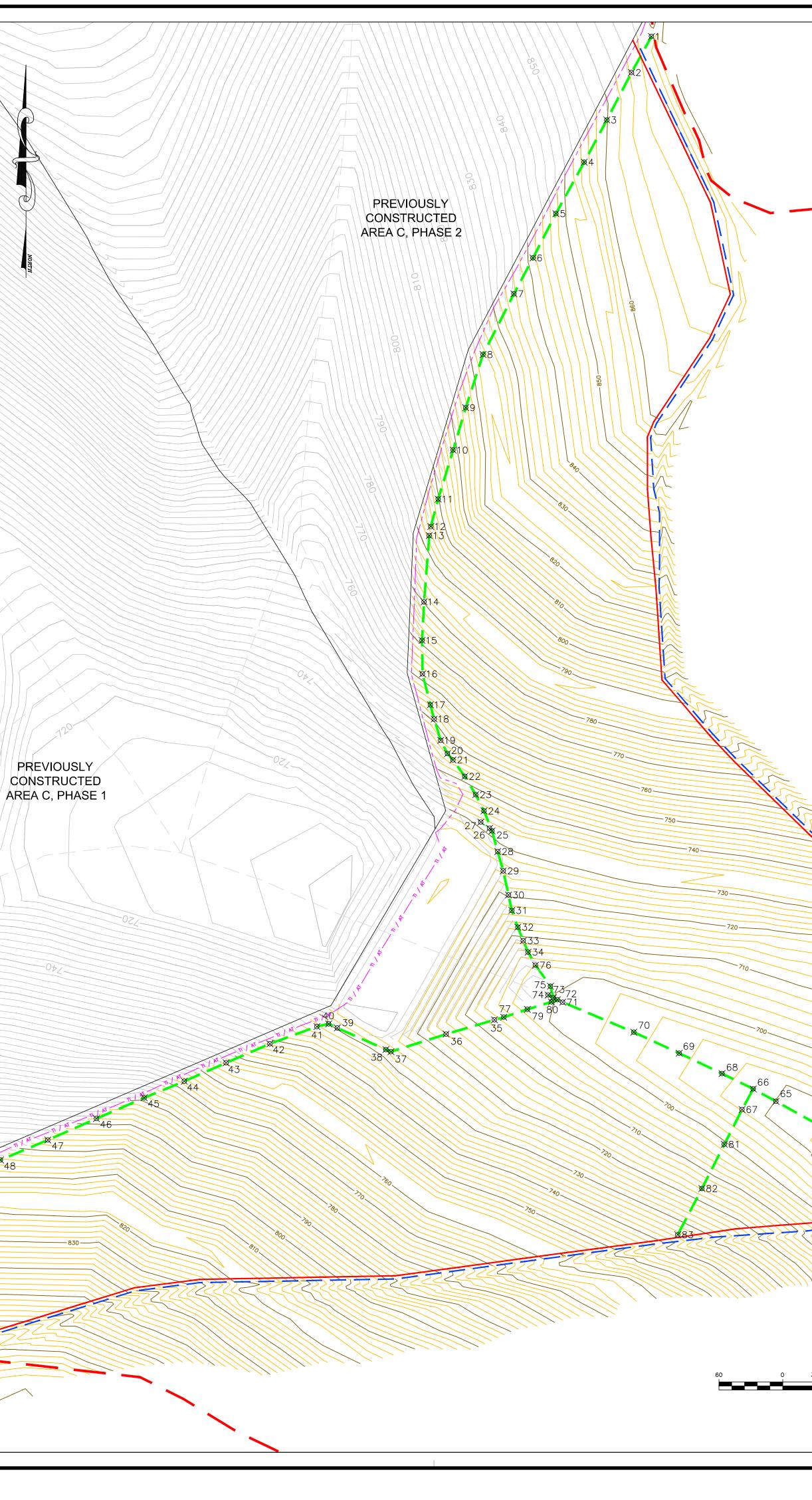
This report is based on data collected and observations made during construction that could be visually seen. Review of design documents and survey information provided by EKPC as well as CQA work performed by Kenvirons based on AECOM's design of Phase 3-B lateral expansion. This post-construction design certification is based on Kenvirons' understanding of AECOM's design plans for the lateral expansion and EKPC's plant operations, maintenance, storm water and CCR handling procedures for the newly constructed lateral expansion. Changes in any of these operations or procedures may result in deviation from the intended design and operation of Phase 3-B.

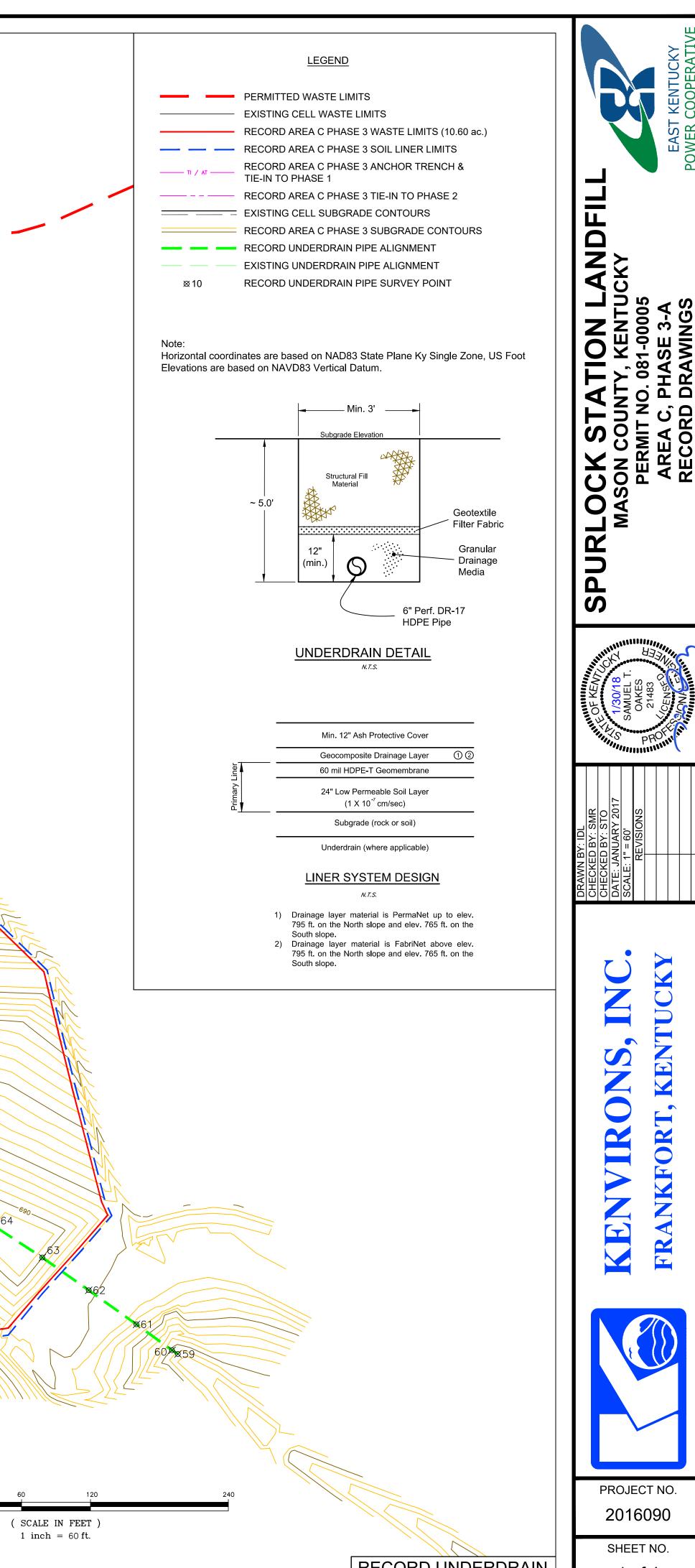
The post-construction certification is based on established engineering principles and provided in a manner consistent with the level of care and skill ordinarily exercised by the engineering consultants under similar circumstances. No other representation is intended.

ATTACHMENT 1

RECORD DRAWINGS

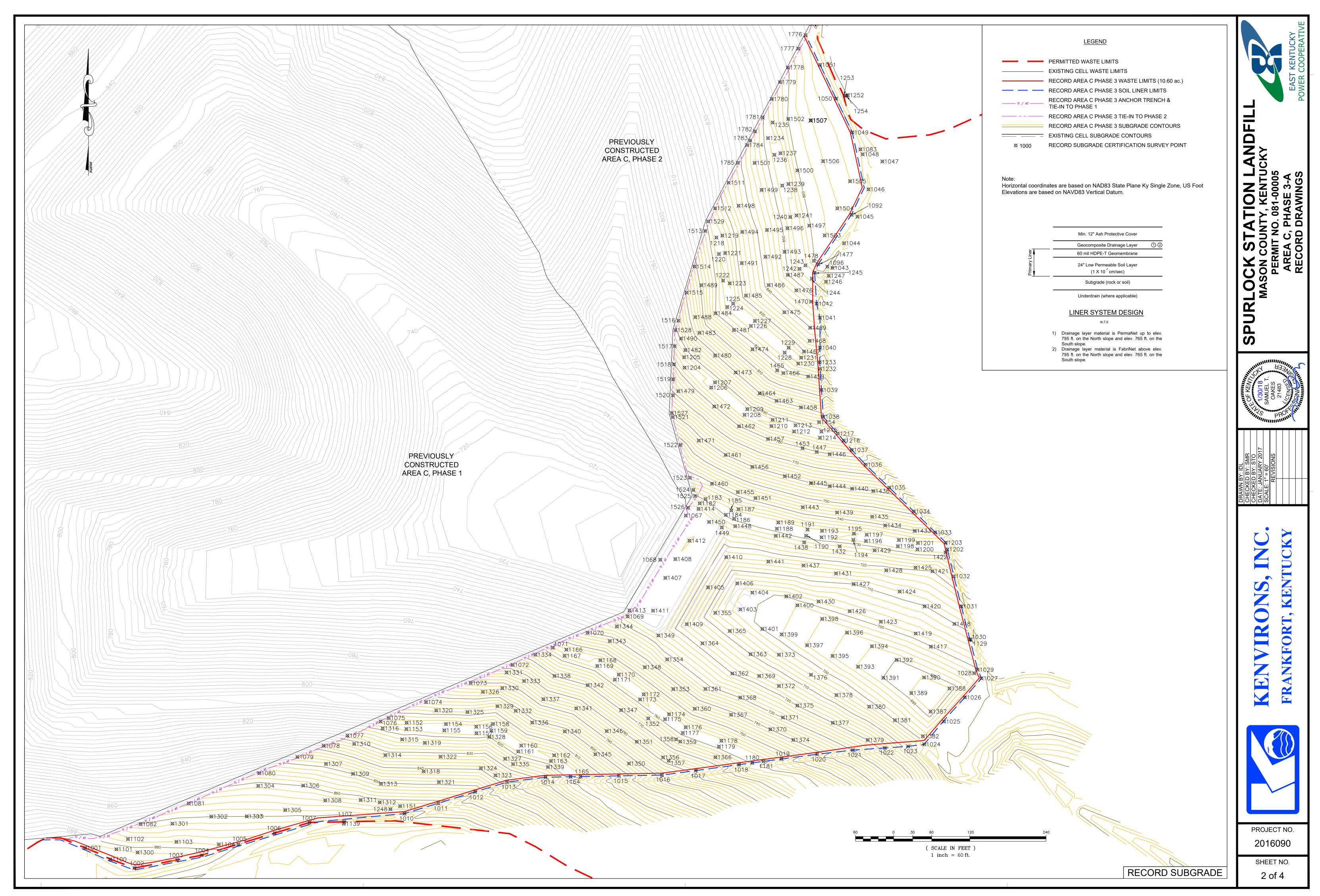
<u>NO.</u>	NORTHING	EASTING	U.D. ELEV.*	DESCRIPTION	PT NO.	NORTHING	EASTING	U.D. ELEV.*	DESCRIPTION	
1	4144998.803	5467968.537	863.59	TOP OF PIPE END	42	4144051.971	5467609.638	736.50	TOP OF PIPE	
2	4144964.939		860.12		43		5467568.465	747.95		$ \sum $
3		5467926.526	855.57		44	4144016.774		761.86		
4 5	4144880.639	5467905.257 5467878.633	850.87 843.55	TOP OF PIPE	45 46	4144001.198 4143981.632		773.19 784.17	TOP OF PIPE TOP OF PIPE	
5 6		5467878.633	843.55	TOP OF PIPE	40		5467446.512	791.50	TOP OF PIPE	
0 7		5467839.155	831.92	TOP OF PIPE	47		5467356.276	791.50	TOP OF PIPE	
8		5467810.125	820.13	TOP OF PIPE	49		5467309.569	807.96	TOP OF PIPE	
9		5467793.848	812.30	TOP OF PIPE	50	4143902.648		816.04	TOP OF PIPE	
10		5467781.982	804.77	TOP OF PIPE	51	4143904.471		814.98	TOP OF PIPE	
11	4144563.973		795.56	TOP OF PIPE	52	4143882.209	5467217.299	827.28	TOP OF PIPE	
12	4144538.186		791.01	TOP OF PIPE	53	4143863.322		839.29	TOP OF PIPE	
13	4144529.695		789.65	TOP OF PIPE	54		5467129.145	847.50	TOP OF PIPE	
14		5467754.647	776.85		55	4143826.349		853.35		
15			767.92		56	4143809.540		856.61		
16		5467753.191	759.10		57		5467005.945	859.40		
17 18	4144370.606 4144357.220		750.61 746.88	TOP OF PIPE	58 59	4143786.015	5466993.933	860.59 676.92	TOP OF PIPE END TOP OF SOLID PIPE END	
19		5467770.268	740.00	TOP OF PIPE	60	4143866.345		678.87	SOLID PIPE BEGIN	
20		5467776.675	740.91	TOP OF PIPE	61	4143888.884		678.50	TOP OF PIPE	
21		5467781.813	740.00	TOP OF PIPE	62		5468210.489	680.80	TOP OF PIPE	
22	4144303.354		737.29	TOP OF PIPE	63	4143947.033		682.80	TOP OF PIPE	
23	4144286.131	5467803.211	732.67	TOP OF PIPE	64	4143973.267	5468130.051	683.96	TOP OF PIPE	
24		5467811.006	728.31	TOP OF PIPE	65	4143997.829		685.86	TOP OF PIPE	
25		5467818.577		TOP OF PIPE TIE IN	66		5468064.231	686.92	TOP OF PIPE TIE IN	
26		5467816.261		TOP OF PIPE TIE IN	67	4143990.172		687.68		
27		5467808.025	728.92	TOP OF PIPE TIE IN	68		5468034.944	688.16		
28		5467823.896	719.75		69		5467994.629	690.20		
29		5467829.109	717.00 713.28		70		5467951.888	692.21		$\langle \rangle \rangle$
30 31		5467834.089 5467837.171		TOP OF PIPE	71 72	4144091.078 4144093.326		694.75 694.98	TOP OF PIPE TOP OF PIPE TIE IN	
31		5467842.792	710.66	TOP OF PIPE	72		5467876.223	695.08	TOP OF PIPE TIE IN	
33		5467847.972	700.03	TOP OF PIPE	73		5467871.318	695.10	TOP OF PIPE TIE IN	
34		5467852.474		TOP OF PIPE	75		5467873.535	695.01	TOP OF PIPE ELBOW	\nearrow
35		5467820.889	701.58	TOP OF PIPE	76		5467859.448	698.40	TOP OF PIPE	-
36		5467775.417		TOP OF PIPE	77		5467829.618	700.13	TOP OF PIPE	740 X
37		5467723.546	722.45	TOP OF PIPE CORNER	79	4144084.374		696.15		$\langle \cdot \rangle$
38		5467718.755	722.68		80		5467874.396	695.23		
39 40	4144066.854	5467673.018	725.05 725.85	TOP OF PIPE TOP OF PIPE CORNER	81 82	4143957.507	5468037.059	696.86 710.84	TOP OF PIPE	-////
40		5467653.844			83		5467993.244	725.79	TOP OF PIPE END	-/////
				820 800						
				820	780					
	800			820 800	780					
				800	780					
	800			800	780					
				800	780					
				800						
				800						
				800					× 51 50	49
				800		11M ¥ 55				49

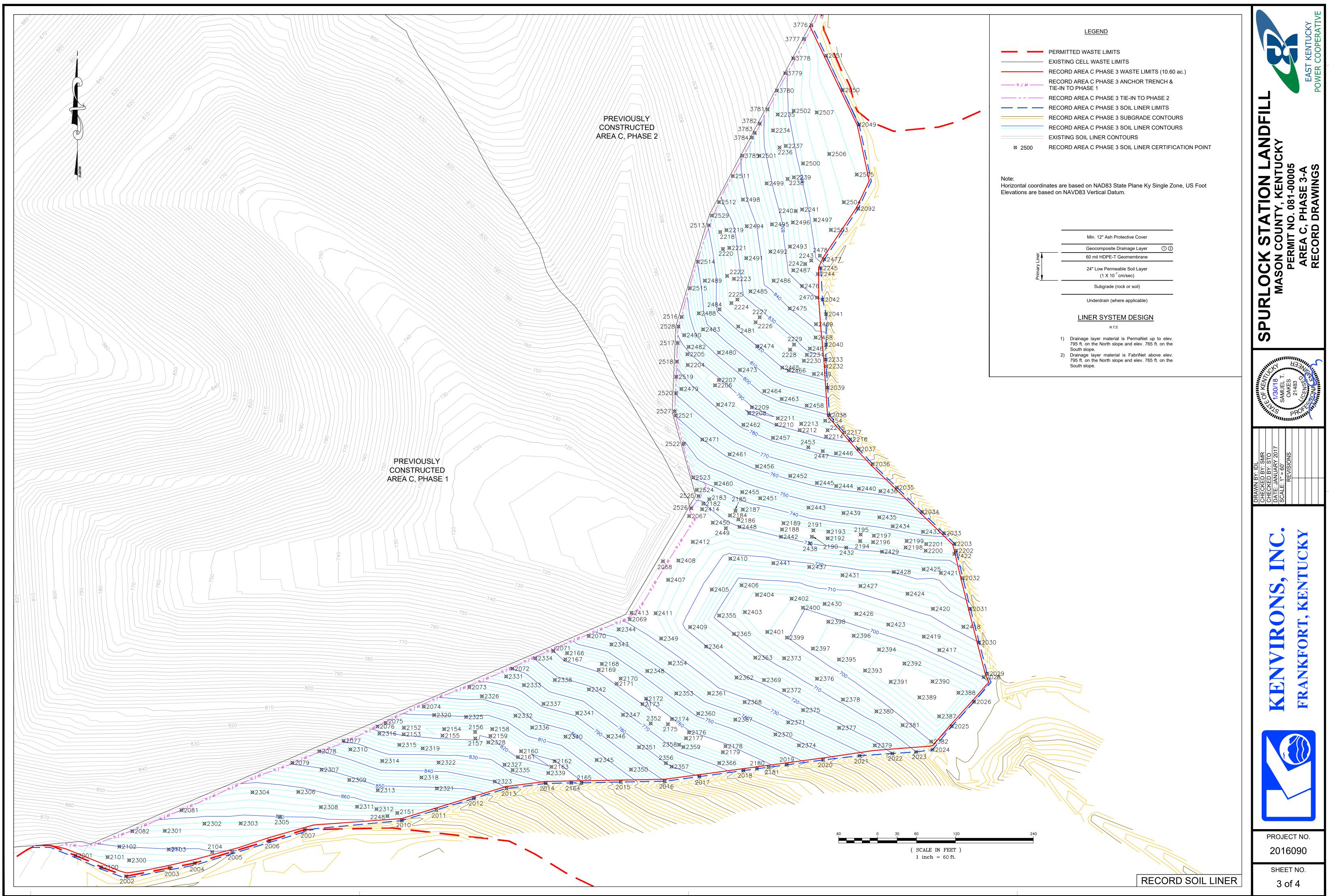




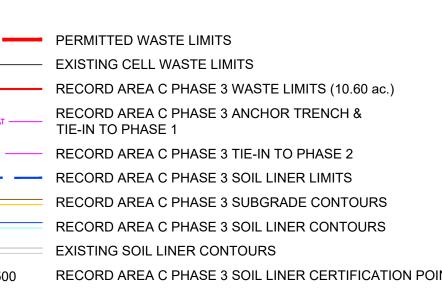
RECORD UNDERDRAIN

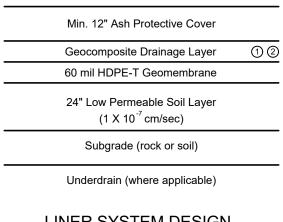
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POINT NUMBER	NORTHING	EASTING	ELEVATION	DESCRIPTION
11221	4144101.16	5467852.63	703.13	leachate collection pipe
11222	4144083.59	5467802.79	711.04	leachate collection pipe
11223	4143847.37	5467879.60	761.79	leachate collection pipe
11224	4143868.44	5467834.13	761.76	leachate collection pipe
11225	4143893.82	5467770.95	761.45	leachate collection pipe
11226	4143929.60	5467721.61	760.70	leachate collection pipe
11227	4143995.82	5467637.14	759.35	leachate collection pipe
11228	4144030.47	5467570.68	757.82	leachate collection pipe
11229	4144242.08	5467837.57	732.52	leachate collection pipe
11230	4144216.79	5467951.79	733.92	leachate collection pipe
11231	4144202.33	5468070.11	735.08	leachate collection pipe
11232	4144187.96	5468179.89	735.81	leachate collection pipe
11233	4144067.88	5467737.09	725.49	leachate collection pipe
11234	4144080.61	5467690.86	727.93	leachate collection pipe
11235	4144038.68	5467566.58	756.70	leachate collection pipe junctio
11236	4144009.35	5467496.54	779.05	leachate collection pipe junctio
11237	4144009.44	5467496.53	779.02	leachate collection pipe
11238	4144232.44	5467789.83	727.23	leachate collection pipe
11239	4144291.79	5467792.73	741.17	leachate collection pipe
11240	4144248.28	5467792.41	729.37	leachate collection pipe
11241	4144104.30	5467864.41	702.64	leachate collection pipe wye
11242	4144058.75	5467964.27	698.37	wye tie-in to existing pipe
11243	4143954.16	5468160.76	690.18	solid pipe/ penetration assemb
11248	4143997.66	5468079.93	694.57	leachate collection pipe
11257	4144190.79	5467819.06	725.33	leachate collection pipe
11263	4144251.50	5467774.13	729.58	tie-in to existing leachate pipe
11318	4144375.814	5467746.696	756.247	top 8" leachate pipe
11319	4144413.23	5467737.41	765.073	top 8" leachate pipe
11320	4144438.673	5467740.237	772.531	top 8" leachate pipe
11321	4144469.555	5467744.261	780.397	top 8" leachate pipe
11322	4144476.185	5467744.564	781.81	top 8" leachate tee
11323	4144477.638	5467746.261	782.813	top 8" leachate pipe
11324	4144478.422	5467747.292	783.194	top 8"leachate pipe connector to
11325	4144479.618	5467745.006	782.584	top 8" leachate pipe
11326	4144508.153	5467746.572	788.697	top 8" leachate pipe
11327	4144533.04	5467750.433	792.352	top 8" leachate pipe
11328	4144564.088	5467757.034	797.831	top 8" leachate pipe
11329	4144480.477	5467750.132	784.482	top 4"leachate pipe
11330	4144482.647	5467759.417	787.49	top 4"leachate pipe
11331	4144479.41	5467771.729	789.669	top 4"leachate pipe
11332	4144463.529	5467792.389	789.631	top 4"leachate pipe
11333	414444.667	5467815.609	789.872	top 4"leachate pipe
11334	4144423.595	5467839.785	789.908	top 4"leachate pipe
11335	4144411.155	5467856.12	790.178	top 4"leachate pipe
11336	4144401.53	5467871.953	790.285	top 4"leachate pipe
11337	4144392.053	5467894.798	790.473	top 4"leachate pipe
11338	4144383.905	5467922.826	790.744	top 4"leachate pipe
11339	4144377.293	5467949.454	791.01	top 4"leachate pipe
11340	4144370.107	5467979.221	791.416	top 4"leachate pipe
11341	4144365.14	5467996.975	791.551	top 4"leachate pipe
11342	4144361.382	5468011.637	791.892	top 4"leachate pipe
11343	4144359.137	5468020.304	790.629	top 4"leachate pipe
11344	4144357.615	5468026.155	788.25	top 4"leachate pipe

